

Self-compacting mortar with sugarcane bagasse ash: development of a sustainable alternative for Brazilian civil construction

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Abstract The technically controlled use of industrial residues in replacement of natural resources for the production of new products with comparable quality presents a great economic and sustainable contribution. In this sense, this work sought to develop self-compacting mortars with partial replacement of sand by sugarcane bagasse ash (SCBA) by a rheological study of the optimum dosage of superplasticizer to cement ratio. The self-compacting parameters were obtained by analyzing the relative flow area and relative flow time, adapting from mortar flow tests. Besides one sample composed only of sand as fine aggregate, the self-compacting mortars presented replacement rates of sand by sugarcane bagasse ash, in mass of 5, 10, 20, 25, 30 and 40%. The results demonstrated that it was

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possible to reduce up to 489 and 56 kg of sand and cement, respectively, to produce 1 m³ of self-compacting mortar with sugarcane bagasse ash considering the expressive replacement rate of 40% of sand by SCBA.

Keywords Self-compacting concrete · Reuse · Rheology · Agro-industrial residues · Sustainability

1 Introduction

Concern for the sustainability of ecosystems is a constant dilemma for modern civilizations. The unbridled extraction of natural resources, triggered during the industrial revolution and intensified with the industrial actions of twentieth and twenty-first centuries, is one of the significant causes of the environmental instability. The residues generated need to be minimized and require more appropriate treatment, reuse and recycling (Nogueira et al. 2012; Moisés et al. 2013).

The self-compacting mortar (SCM) offers several benefits associated with the product and the operational processes, both in precast plant operations and in direct applications at construction sites. The developments of by-products associated with self-compacting concrete (SCC), such as the paste and SCM, are also fundamental for the determination of the self-compacting properties of the self-leveling concretes (Safi et al. 2013; Chatterjee and Das 2013). In this context, there are a lot of studies that have developed SCM with the most varied materials (nanoparticles, industrial residues as the aluminum, among others) to improve the characteristics of resistance, workability and consumption, proposing at the same time, the use of new components in a technical and controlled way (Rao et al. 2015; Silva and Brito 2015; Mohseni et al. 2015; Madandoust et al. 2015).

The big challenges of these developments, whether for the paste, the mortar or even the concrete, are linked to the rheological aspects related to the establishment of fluidity, cohesion and segregation resistance. It is important to emphasize that each of these by-products requires specialized investigations with appropriate tests and techniques (ABNT NBR 15823-1 2017; Gomes and Barros 2009; EFNARC 2005; Okamura and Ouchi 2003).

Various dosage methods of SCC use the development of SCM as basis to guarantee the self-compatibility in the fresh state and the required mechanical resistances in the final state (Gomes 2002). The Okamura and Ouchi (2003) method seeks to establish parameters for the study of these properties: the relative flow area (Γ_m), assumed as G_m in this research, and the relative flow time (R_m), which measures the deformability by mortar flow test and the viscosity by mortar funnel test, respectively. Based on the method of Nepomuceno et al. (2012), Silva and Brito (2015) attested the importance of SCM for the optimized development of SCC, both for results in fresh and hardened state.

The sugarcane bagasse ash (SCBA), which is the industrial residue used in this research as a fine aggregate, as well as the sand, is classified by the ABNT NBR 10004 (2004) as rural residue belonging to class II and not inert. The SCBA comes from the process of energy cogeneration in boilers by burning the sugarcane bagasse. Based on data from CONAB (2017) and FIESP/CIESP (2012), for the 2017/2018 harvest, up to 240,000 ton of SCBA can be produced just in the state of Paraná (Brazil), if all the bagasse generated by the production of sugarcane is reused in processes of energy cogeneration. The potential of SCBA generation is much more significant when these same indicators allow estimating

that the expected 646.34 million tons of sugarcane, in the 2017/2018 Brazilian harvest, would present the potential for production of up to 4008,000 tons of SCBA.

The SCBA generated in processes of energy reutilization, from the bagasse burning in the boilers and furnaces, present heterogeneous characteristics in the physical aspects (related to form, size and coloration), as well as in the variability of the chemical composition. These characteristics are related to the variations of the internal burning conditions, generated by the temperature profile in the boiler extension. In addition, other aspects of the generation process act in the unadjusted formation (e.g., the internal combustion atmosphere, the gas treatment and washing system, the subsequent treatment process and the ashes storage) for both light ashes, from gases and particulates in suspension, and heavy ashes, which are burnt from the bottom of the boiler. Furthermore, the inherent variations in the characteristics that precede the burning process such as harvesting technology (manual or mechanical), the way of extraction of the broth in the initial process that results in the generation of the bagasse with the possibility of encompassing other parts of sugarcane, the moisture of the bagasse, which is usually deposited in uncovered areas, and the variety of sugarcane and the soil where it was cultivated must be considered. The considerations of such factors in SCBA generation motivated several researchers to develop the application of SCBA in the civil construction as substitute of fine aggregates for structural filling. Sales and Lima (2010) partially replaced sand by SCBA obtaining better mechanical results than concrete reference samples. Modani and Vyawahare (2013), Vanderlei et al. (2014) and Moretti et al. (2016) presented results on the use of SCBA in the substitution and composition with sand to produce mortars and concretes. Faria et al. (2012) and Zhang (2013) developed compositions in which SCBA was used as an aggregate in the blending of materials to produce bricks.

Some researchers have developed additional controls of reburning, processing and grinding at laboratory scales for SCBA generation with chemical and physical characteristics suitable to pozzolanic investigations, such as Crespi et al. (2011) and Bahurudeen and Santhanam (2015).

In general, the current manuscript aimed to develop self-supported mortars with substitution rates of sand by SCBA. The rheological analysis of G_m and R_m parameters was carried out to classify the characteristics of the final material. Finally, investigations were elaborated by means of RSM package of R software (R Development Core Team 2016) to estimate the parameters of mathematical models of second-order response surface.

2 Materials and methods

The physical characterization of materials, specially granulometric profiles of sand and SCBA, were performed according to ABNT NBR 7181 (2016) and ABNT NBR 7211 (2009a, b). The SCBA was collected from Iguatemi Sugarcane Plant (Maringá, Brazil) of Usaçúcar Group. The chemical composition of SCBA was determined by X-ray fluorescence (XRF), and the patterns of SCBA were verified by X-ray diffraction (XRD). The analytical LOI values of the sample were determined by comparing the weight before and after ignition for 240 min at 600 °C.

The development of SCMs was based on the composition of the self-compacting paste prepared by Molin Filho et al. (2011), which did not contain the SCBA. The self-compacting parameters of the mortar were investigated by technical adaptations of the mini-conical slump

flow test, performed with the mini-cone for mortars, which was based on the techniques used by Okamura and Ouchi (1999) and (2003).

SCMs were composed of cement (CP II E 32), calcitic limestone filler, third-generation superplasticizer chemical additive (Glenium 51—Manufacturer Basf), quartzose sand, crystalline SCBA (burning above 700 °C) and water in compliance with ABNT NBR 15900-1 (2009a, b).

The preparation, production and tests with the mortars were carried out with a mortar mixer of 5 L in a speed of 60 ± 5 rpm, a glass plate with a mini-cone (Fig. 1a), a thermometer, a chronometer, a precision scale of $2000 \text{ g} \pm 0.01$ and a bench level.

The preparation, mixture and pouring stages were performed as described by Molin Filho et al. (2011) with the addition of sand and SCBA in the initial of the preparation. It is highlighted that during the experiments, the temperature of the mortars maintained around 21 ± 3 °C with relative air humidity between 45 and 60%.

The used mini-cone had a bottom diameter (d_0) of 125 mm and height of 63 mm. In the method, the final flow area (d) was calculated with the average of two perpendicular measurements and was used to obtain the G_m according to Eq. (1).

$$G_m = \left(\frac{d}{d_0} \right)^2 - 1.0 \quad (1)$$

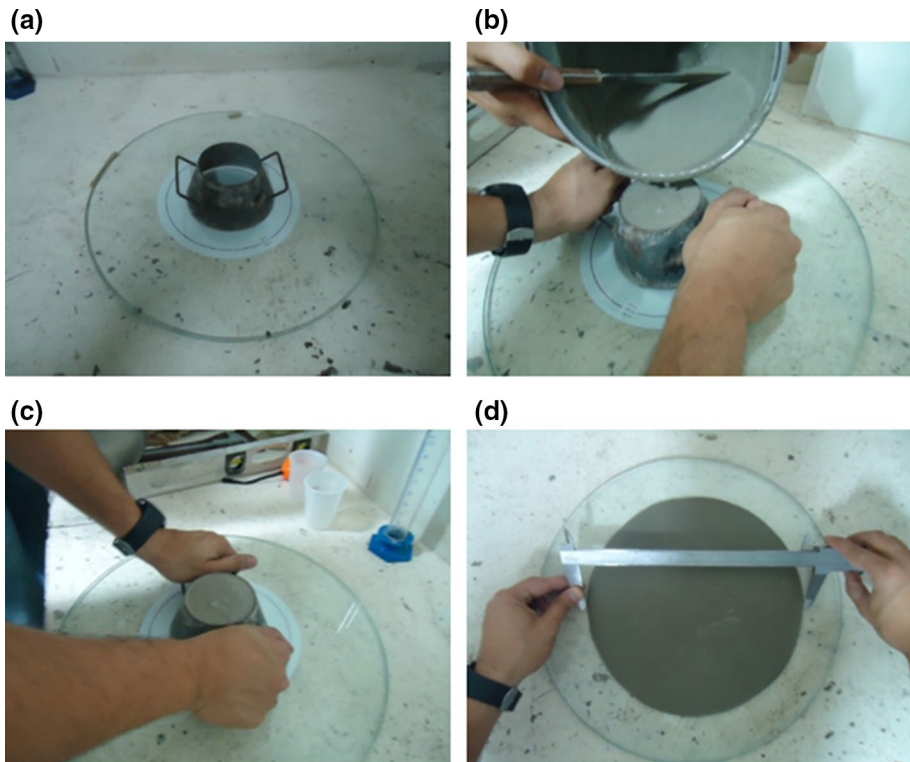


Fig. 1 Images of the apparatus and the test of mini-cone for mortars. **a** Mini-cone and the glass plate of 400 mm diameter; **b** mortar pouring; **c** leveled mortar inside the mini-cone; **d** appearance of the mortar after stabilization of the movement and measurement of the flow area diameter

G_m = Relative flow area, expected value of 5 ± 0.5 , d_0 = initial diameter of the mixture (mm), d = average of two perpendicular measurements of the flow area (mm).

Similarly, it was performed an adaptation of Okamura and Ouchi (2003) method to calculate the R_m . In these adaptations, as shown in Eq. (2), instead of using the V-Funnel for mortars, the flow time (t) was obtained by the measurement of the time between the time zero of the flow and the final stabilization of the volume of the mini-cone in the plate. Therefore, an experimental proposal was assumed to obtain SCMs. It is important to emphasize that the obtained SCMs will be used as basis to develop compositions of SCC, which require more elaborated and normalized experiments to measure parameters as the viscosity and the fluidity.

$$R_m = \frac{10}{t} \quad (2)$$

R_m = Relative flow time, expected value of 1 ± 0.1 , t = mortar flow time (s).

For these tests, Edamatsu et al. (1999) suggested a final diameter of the flow area of 200–283 mm ($3 \leq G_m \leq 7$) and flow time of 5–10 s ($1 \leq R_m \leq 2$). According to Takada and Walraven (2001), to produce SCC, the G_m and R_m values need to be of 5.0 and 1.0, respectively. These are referential numbers to obtain the content of superplasticizer in the paste and the consequent determination of water/filler (w/f) and water/cement (w/c) ratios. Nunes (2001) reported that within the allowable ranges, higher values of G_m provide mortars with higher deformability, while lower values of R_m are characteristic of mortars with high viscosities. As mentioned in Eqs. (1) and (2), the criteria for verifying the optimum dosage condition of superplasticizer in the mortars considered $G_m = 5 \pm 0.5$ and $R_m = 1 \pm 0.1$. Based on Edamatsu et al. (1999), as a further reference, it was estimated that the optimum SCMs would reproduce flow areas with an average diameter “ d ” within the range of 300 ± 30 mm, by the volume of the used mini-cone in comparison with the mini-cone of Okamura and Ouchi (2003).

The experimental challenge to obtain these SCMs was to incorporate the sand (sand/cement [s/c] ratio of 2), in the same way as shown by Gomes (2002), with the projected replacements of sand by SCBA, to the trace of Molin Filho et al. (2011) paste (f/c ratio of 0.4 and w/c ratio of 0.5). Therefore, at first, SCM to be tested assumed the paste dosage of Molin Filho et al. (2011) only with the sand as fine aggregate. Subsequently, the replacement ranges of 5, 10, 20, 25, 30 and 40% of sand by SCBA were assumed for the analyses. For each composition, the G_m and R_m parameters were evaluated. To obtain the optimum value for each replacement range, simulations with sp/c values around the optimum relation of the previous replacement range were performed, by means of a progressive and constructivist experimental method. The cement consumption for the mortar volume of 1 m^3 was obtained by Eq. (3). The specific mass was calculated by the measurement of the mortar mass in a cubic container of 0.001 m^3 previously calibrated.

$$M_c = \frac{\rho_{scm} \times V_{scm}}{1 + w/c + f/c + sp/c + s/c + SCBA/c} \quad (3)$$

M_c = mass of cement in kg per 1 m^3 , ρ_{scm} = SCM specific mass in kg/m^3 , V_{scm} = SCM volume in m^3 , w/c = water/cement ratio, f/c = filler/cement ratio, sp/c = superplasticizer/cement ratio, s/c = sand/cement ratio, $SCBA/c$ = SCBA/cement ratio.

After calculating the mass of cement, it was possible to obtain the consumption of all the other materials for 1 m^3 of mortar by applying the final factors obtained in relation to the cement. The optimum compositions of G_m and R_m were mathematically modeled

in second-order configurations by means of the response surface methodology (RSM) of R software (R Development Core Team 2016). The response surface and contour graphs were plotted to evaluate the predictions of the models versus the experimental values.

3 Results and discussion

3.1 Physical and chemical characterization of SCBA

The sand had specific mass of 2642.5 kg/m^3 , maximum diameter of 0.6 mm and fineness modulus of 1.7. The used SCBA, from Iguatemi Sugarcane Plant (Maringá, Brazil) of Usaçúcar Group, was from the same sample of Moisés et al. (2013) studies. It had a specific mass of 2640 kg/m^3 and the highest proportion of its particles, around 73%, had a size between 150 and 250 micrometers. It is important to emphasize that the SCBA was used in the natural condition, being only sieved in the mesh 0.595 mm (#30), to remove the organic impurities (Fig. 2).

Figure 3 shows sand and SCBA granulometric profiles characterized by ABNT NBR 7181 (2016) and ABNT NBR 7211 (2009a, b).

The percentage of particles with a maximum dimension of 1.2 mm is similar between the two materials, as shown in Fig. 3. However, the proportions are very divergent for particles with dimensions smaller than 0.42 mm. While about 99.4% of the SCBA particles are less than 0.42 mm, only 33.5% of the sand particles are $< 0.42 \text{ mm}$. This difference is even more significant for particles that are smaller than 0.25 mm, in which about 80% of the SCBA particles and 0.3% of the sand particles are in this size range. Moreover, only 3.6% of SCBA particles are less than 0.075 mm. This information corroborates with the indication for the use of SCBA in substitution of the sand in its raw state (without grinding). According to ABNT NBR 5752 (2014), for a material be evaluated as pozzolanic material in substitution to the Portland cement, at least 80% of its particles need to be smaller than 0.045 mm.

Fig. 2 Visual aspect of a sample of sugarcane bagasse ash (SCBA)



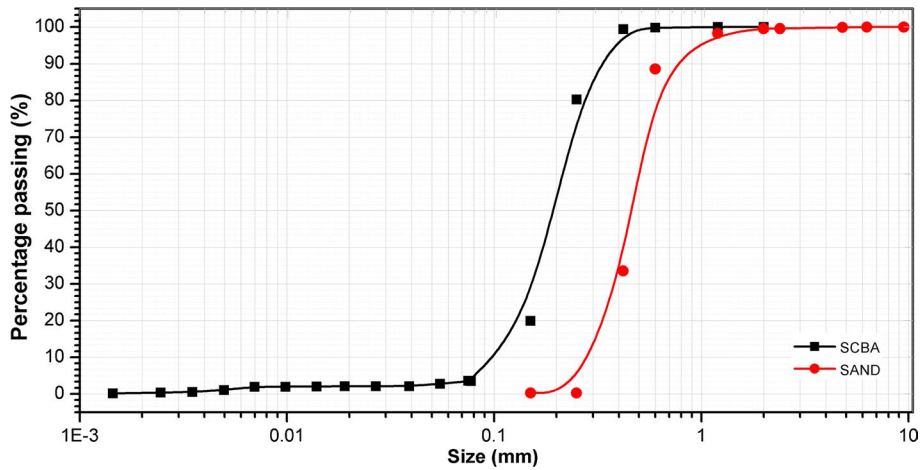


Fig. 3 Sand and SCBA granulometric profiles

Table 1 shows the SCBA composition studied by X-ray fluorescence (XRF).

In Table 1, it is possible to observe the predominant presence of silicon dioxide (SiO_2), in accordance with results of Moisés et al. (2013). Figure 4 presents the X-ray diffraction (XRD) pattern of the SCBA. The results indicated that the sample is crystalline with an α -quartz structure (standard pattern number 201354—ICSD database) (Lager et al. 1982). The loss on ignition (LOI) was 0.9%.

The physical and chemical characteristics of the crude residue investigated in this research have provided the indication for an application of SCBA as an aggregate for structural filling. To do that, only the separation of particles by sieving (incomplete burning fibers and organic material) was needed. The prospect of promoting an application that requires the incorporation of the residue into new products with the lowest energy after its conventional process of generation was achieved. This has led to concerns about the development of a responsible social and environmental destination for SCBA and to a low cost of preparation. The combination of these aspects is fundamental in the generation of added value regarding the destination of SCBA and consequently fomentation of the interest of the sugarcane plants in the accomplishment of this solution.

Table 1 X-ray fluorescence (XRF) results of SCBA composition

Elements	Mass (%)
SiO_2	97.9
Al_2O_3	0.3
K_2O	0.1
CaO	0.1
Fe_2O_3	1.2
TiO_2	0.3
Other elements	0.1

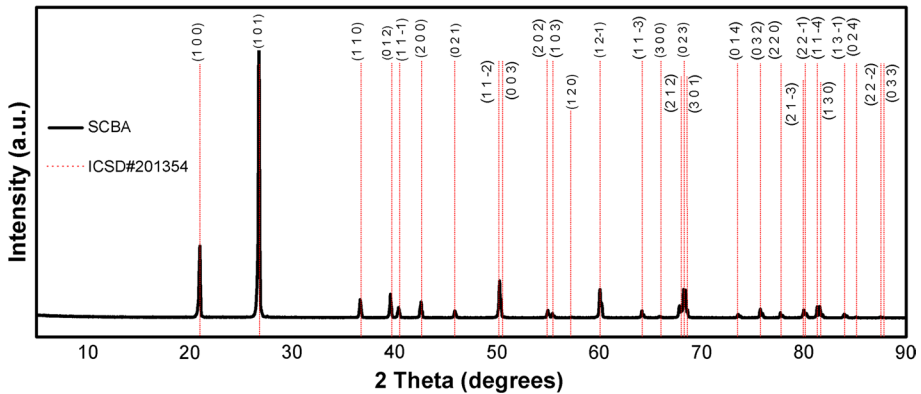


Fig. 4 X-ray diffraction (DRX) spectrum of SCBA

3.2 Development of the self-compacting mortars compositions

Table 2 presents the composition and the G_m and R_m values of all the studied samples to carry out the analysis of the optimum dosage of sp/c for all the replacements of sand by SCBA. In total, 26 samples of mortars numbered from 0 to 25 were evaluated in this stage.

The mortar “0” was initially developed as a reference for all the subsequent samples. This mortar had the same unitary composition of Molin Filho et al. (2011) paste with the difference of the addition of the s/c ratio = 2 (100% of sand). It presented satisfactory results, because with the same dosage of sp/c = 0.004 of the paste, it was possible to obtain the expected G_m and R_m values based on the requirements taken into consideration in this research for SCM. Therefore, these results showed that this sample could serve as a fundamental reference for SCMs, although still without SCBA.

It is also worth noting that the rows marked in bold in Table 2 represent the best combinations of materials by the G_m and R_m tests in each rate of SCBA/s. It is also possible to observe that the complementary criterion that estimated the average diameter “ d ” of the flow area was met, where all of them were within the allowable values of this method. In this stage, mortars with SCBA were developed with potential for production of SCC. It was noted that the increasing presence of SCBA in the mixture would require additions of superplasticizer to be preserved the expected characteristics of fluidity, homogeneity and cohesion. The ranges of replacement of sand by SCBA from 5 to 20% showed that, up to this variation, it is possible to maintain a consumption of sp/c very close to the consumption of the mortar that contain 100% of sand, i.e., sp/c of 0.0042 (+5%). The replacements of 25, 30 and 40% demanded an increase in consumption, with respective values of 0.45, 0.5 and 0.58% in relation to the mass of the cement. Based on Eq. (3), the material consumptions for each cubic meter of mortar were calculated and are shown in Table 3.

All the material consumption of the optimum mortars, presented in Table 3, are within allowable ranges to produce SCCs because the volume of mortar in these SCCs should be up to 70% of the mixture and the volume of coarse aggregates should be about 30% (EFNARC 2005; Gomes 2002). In this condition, the cement consumption for 1 m³ of concrete originated from SCM 0, which requires the higher amount of cement, is around 370 kg, and for SCM 25 is around 331 kg. Consequently, when it considers the fines dosage [cement + filler + 27% of the mass of the SCBA that is < 0.150 mm (EFNARC 2005)],

Table 2 SCM compositions and test results

Tested compositions										Test results			
SCM	sp/c (%)	SCBA/s (%)	SCBA (g)	Cement (g)	Filler (g)	Sand (g)	Additive (g)	Water (mL)	G _m	Time (s)	R _m	Flow area (mm)	Specific mass (kg/m ³)
0	0.40	0	0	700	280	1400	2.8	348	5.27	10.7	0.93	313	2271
1	0.30	5	70	700	280	1330	2.10	350	1.03	2.84	3.52	178	2145
2	0.35	5	70	700	280	1330	2.45	349.7	3.24	6.45	1.55	258	2180
3	0.40	5	70	700	280	1330	2.80	349.5	3.41	7.8	1.28	263	2195
4	0.45	5	70	700	280	1330	3.15	349.3	5.86	10.6	0.94	328	2214
5	0.42	5	70	700	280	1330	2.94	349.4	4.95	10.6	0.94	305	2201
6	0.35	10	140	700	280	1260	2.45	349.6	3.53	5.86	1.71	266	2158
7	0.40	10	140	700	280	1260	2.80	349.4	4.76	8.37	1.19	300	2222
8	0.45	10	140	700	280	1260	3.15	349.1	5.23	11.2	0.89	312	2192
9	0.42	10	140	700	280	1260	2.94	349.4	5.11	10.6	0.94	309	2200
10	0.35	20	280	700	280	1120	2.45	349.5	3.08	6.24	1.60	253	2168
11	0.40	20	280	700	280	1120	2.80	349.3	3.49	8.88	1.13	265	2181
12	0.45	20	280	700	280	1120	3.15	349	5.80	11.5	0.87	326	2202
13	0.42	20	280	700	280	1120	2.94	349.1	4.57	10.8	0.93	295	2180
14	0.35	25	350	700	280	1050	2.45	349.5	2.76	7.5	1.33	243	2116
15	0.40	25	350	700	280	1050	2.80	349.2	3.23	9.1	1.10	257	2120
16	0.42	25	350	700	280	1050	2.94	349.1	3.68	9.58	1.04	271	2108
17	0.45	25	350	700	280	1050	3.15	349	4.78	10.0	1.00	301	2194
18	0.40	30	420	700	280	980	2.80	349.1	3.03	7.23	1.38	251	2150
19	0.45	30	420	700	280	980	3.15	348.9	3.82	9.57	1.04	275	2149
20	0.50	30	420	700	280	980	3.50	348.6	4.68	10.5	0.95	298	2109
21	0.45	40	560	700	280	840	3.15	348.7	1.09	5.5	1.82	181	1998

Table 2 (continued)

Tested compositions										Test results			
SCM	sp/c (%)	SCBA/s (%)	SCBA (g)	Cement (g)	Filler (g)	Sand (g)	Additive (g)	Water (mL)	\bar{G}_m	Time (s)	R_m	Flow area (mm)	Specific mass (kg/m ³)
22	0.50	40	560	700	280	840	3.50	348.5	2.64	8.47	1.18	239	2047
23	0.55	40	560	700	280	840	3.85	348.2	3.10	9.89	1.01	253	2073
24	0.60	40	560	700	280	840	4.20	348	5.45	11.4	0.88	318	2119
25	0.58	40	560	700	280	840	4.06	348.1	5.03	10.2	0.98	307	2116

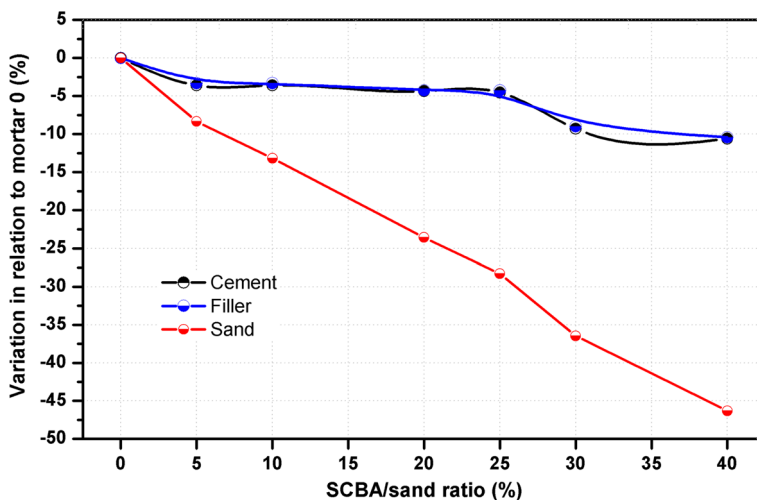
Table 3 Material consumption for 1 m³ of mortar

Components (kg)	SCM 0	SCM 5	SCM 9	SCM 13	SCM 17	SCM 20	SCM 25
	SCBA 0%	SCBA 5%	SCBA 10%	SCBA 20%	SCBA 25%	SCBA 30%	SCBA 40%
Cement	528	509	509	505	504	479	472
Filler	211	204	204	202	202	192	189
Sand	1056	968	917	807	757	671	567
SCBA	0.0	51	102	202	252	288	378
Water	264.1	254.7	254.6	252.3	252.2	239.7	236.2
Superplasticizer	2.11	2.14	2.14	2.12	2.27	2.40	2.74

the sum of the values for these two mortars, SCM 0 and SCM 25, would provide for the concrete with 70% of volume of SCM, the values of 517 and 534 kg of fines consumption, respectively. These values are within the recommended ranges of EFNARC (2005), from 380 to 600 kg/m³.

Figure 5 shows the variations in percentage of consumption of dry components in relation to mortar “0” considering the consumptions related in Table 3 and the consumptions dimensioned by Eq. (3) as parameters. In the same sense, Fig. 6 shows the variation for the liquid components. Such variations are inherent to the differences in consumption due to the specifications of the dosages in relation to the cement, and by the respective variations of the specific mass obtained experimentally.

All the materials showed projections for the reduction in the consumption with the increment of higher rates of SCBA/sand, as shown in Fig. 5. The cement and filler had reductions in close percentages at all SCBA/sand rates, reaching maximum reduction values of about 11% for cement and filler at SCM 25, which potentiates the reduction of 56 and 22 kg of each component per 1 m³, respectively. Although the experimental replacement of SCBA/sand for SCM 25 was performed with 40% due to the decrease of

**Fig. 5** Variations of the consumption of dry components in relation to the reference mortar

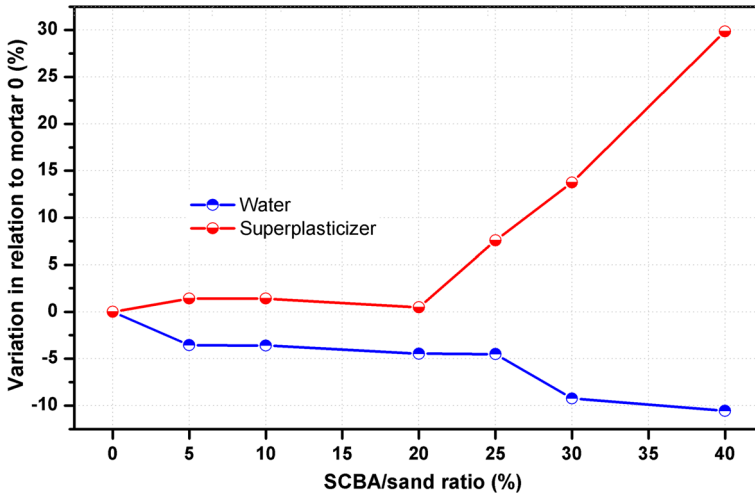


Fig. 6 Variations of the consumption of the liquid components in relation to the reference mortar

0.155 kg in the specific mass, the reduction in the consumption of 46% of sand (489 kg) in 1 m³ of SCM was possible in direct comparison with the reference SCM “0.” In other replacement levels, this considerable reduction of sand was also evidenced, as the decrease of 249 kg (24%) for the replacement rate of 20%.

Figure 6 shows a decrease in water consumption, which contrasts with the increase in consumption of superplasticizer. To guarantee the expected workability in SCMs with higher SCBA/sand dosages, it is necessary to increase the sp/c ratio. However, at the same time, it is possible to reduce the water projection in SCM. This factor contributed directly to the experimental effect of the decrease in the specific mass in relation to SCM “0,” for all the mortars developed with contributions for projections of lower component consumption. Besides that, the use of SCBA provided an improvement in the microstructural filling as an additional factor. The packing of the fine aggregates, due to the granulometric differences (see Fig. 2), resulted in more distributed plots for particles smaller than 0.25 mm and larger than 0.075 mm. This range represents more than 75% of SCBA particles. Therefore, a percentage of these spaces, previously occupied by fines (filler and cement) and water, began to be occupied by SCBA particulates.

Figures 5 and 6 show that, apart from superplasticizer, it is possible to produce self-compacting mortar with up to 40% of sand replacement by SCBA. To maintain the cohesion and fluidity aspects expected in the G_m and R_m tests, it was necessary to assume percentages of sp/c greater than the reference trace from the SCBA/sand replacement rate of 20, up to 30% higher for SCM 25. However, all other components of natural origin presented reductions in their actual consumption, in which this practice specially makes it possible to assume a responsible and effective technical allocation for SCBA.

Considering the Brazilian potential of SCBA generation, it would be possible to apply SCM 25 in civil construction materials saving more than 4.008.000 tons of sand to produce up to 8.196.319 cubic meters of SCM. If this volume of SCM was applied in the production of SCC, with the minimum volume of mortar of 63% (EFNARC 2005), it would be possible to obtain about 13.010.030 m³ of concrete. This volume of SCC

would be sufficient to build more than 160 similar stadiums to Maracanã, or even so, a hydroelectric power plant like Itaipu (Santos 2011; Itaipu Binacional 2017).

3.3 Adjustment of the response surface models

The parameters of second-order models (SO) were adjusted for the representations of the response surfaces by means of RSM package of R software (R Development Core Team 2016). Table 2 is used as basis to obtain Table 4 with the coded data of the model, by a representation based on the analysis of a central composite design (CCD), fixed in the central range of 20% (SCM 13). Due to the software inputs format, the $\times 1$ and $\times 2$ variables of the model were renamed to “SCBA” (which represents the SCBA/s rates) and “Super” (which represents sp/c ratio), respectively, and the responses were defined as G_m and R_m .

Table 4 uses the optimum experimental results of dosage for all the ranges of SCBA/s analyzed, with the mortar with 20% (SCM 13) as center point. The optimum mortars with replacement of 5% (SCM 5) and 25% (SCM 17) were not used because they were not equidistant from the central reference. To extend the analysis points, it was used the concept of distribution of the central composite rotational design (CCRD), with the coordinated points (SCBA, Super): (0, 1.41), (0, -1.41), (1.41, 0) and (-1.41, 0). The G_m and R_m values for these points were assumed by extrapolation of the optimum values of the seven SCMs. This configuration has central, axial and rotational points, which are characteristic for the adjustment of the second-order models. The coefficients of the model were estimated with RSM library and are depicted in Table 5.

As given in Table 5, it is possible to present Eq. (4) and Eq. (5), which describe the second-order response surface models for G_m and R_m , for the predictions of the coded values.

$$\hat{G}_m = 4.23 + 1.75\text{SCBA} - 1.89\text{Super} - 1.64\text{SCBA}^2 - 1.80\text{Super}^2 + 4.38(\text{SCBA} \times \text{Super}) \quad (4)$$

$$\hat{R}_m = 0.95 - 0.04\text{SCBA} + 0.06\text{Super} + 0.12\text{SCBA}^2 + 0.06\text{Super}^2 - 0.17(\text{SCBA} \times \text{Super}) \quad (5)$$

All the coefficients generated by the models can provide reliable results with at least 99% of confidence, which is corroborated by the p value (α) of the two models, 6.262e-06 for G_m and 0.001189 for R_m . Figure 7 shows the perspective of the predicted values versus observed values for G_m and R_m . The values of the coefficient of determination R^2 of 0.9998 for G_m and of 0.995 for R_m show the existence of an optimum numerical correlation between the observed and predicted values in both models.

Table 4 Coded data for the model studies

Reference	SCBA (x_1)	Super (x_2)	G_m	R_m
SCM 0	-1	-1.00	5.27	0.93
SCM 9	-0.5	-0.78	5.11	0.94
SCM 13	0	-0.78	4.57	0.93
SCM 20	0.5	0.11	4.68	0.95
SCM 25	1	1.00	5.03	0.98
Rotational	0	1.41	3.43	1.14
Rotational	0	-1.41	-1.52	1.24
Rotational	1.41	0	-2.04	1.15
Rotational	-1.41	0	3.30	0.99

Table 5 Coefficient estimates (output synthesis of R *software*) for G_m and R_m

Coefficients for G_m			Coefficients for R_m		
	Estimate	Pr(> t)		Estimate	Pr(> t)
(Intercept)	4.229204	3.383e-06 ***	(Intercept)	0.9453747	3.971e-06 ***
SCBA	1.747056	5.929e-06 ***	SCBA	-0.0361701	0.0080348 **
Super	-1.887265	4.464e-06 ***	Super	0.0566909	0.0020866 **
SCBA:Super	4.375029	2.966e-06 ***	SCBA: Super	-0.1708267	0.0006433 ***
SCBA ²	-1.639109	P1.654e-05 ***	SCBA ²	0.1218484	0.0005214 ***
Super ²	-1.800014	1.396e-05 ***	Super ²	0.0621792	0.0042047 **
Multiple R^2 : 0.9998			Multiple R^2 : 0.995		
Adjusted R^2 : 0.9996			Adjusted R^2 : 0.9867		
F-statistic: 3989 on 5 and 3 DF			F-statistic: 119.8 on 5 and 3 DF		
p value: 6.262e-06			p value: 0.001189		

Signif. codes: 0 '***' 0.001 '**' 0.01 '*' 0.05 '.' 0.1 ' ' 1

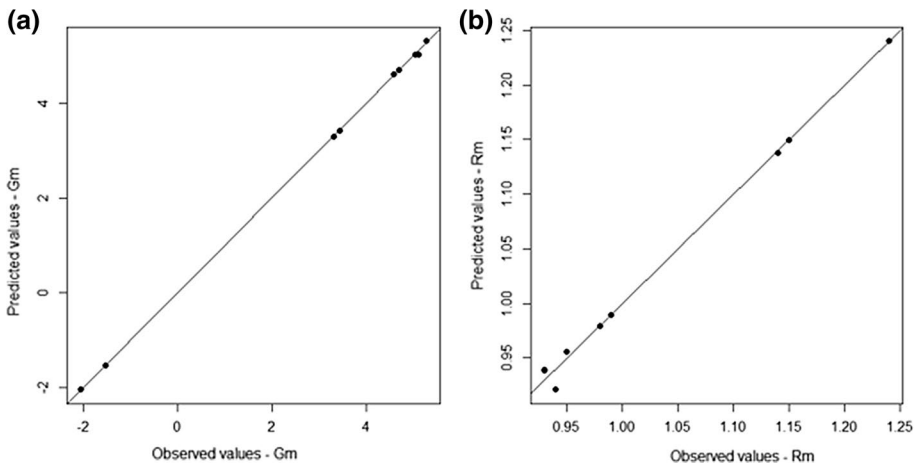


Fig. 7 Graphical comparison of predicted and experimental values of G_m and R_m . (a) Graph of experimental values versus predicted values for G_m ; (b) graph of experimental values versus predicted values for R_m

The response surfaces graphs and their respective contours were generated in view of the information that validates the models (Fig. 8).

In Fig. 8, by the predictions that can reproduce the expected yields in the G_m (5 ± 0.5) and R_m (1 ± 0.1) tests, it can be visualized that there are good amplitudes of combinations of SCBA/s and sp/c. Besides that, it is important to emphasize that it is possible to reproduce compositions of SCMs in all replacement ranges of SCBA/s, with increasing consumption of sp/c as the SCBA is added, something that had already been noticed in the experiments.

Through the generated contour graphs, it is possible to verify that there are broad central bands that would simultaneously represent satisfactory results of G_m and R_m . Furthermore, it can be noticed that the optimum points of yields (maximum) are close to the center of the graphs, slightly shifted down.

The maximum performance of G_m , by adjusting the response surface model in R software, corresponds to the point of coordinates (0.27; -0.20). It represents effectively the SCBA/s ratio of 25.4% and sp/c ratio of 0.47 with the predicted value of 4.65 for G_m . It is also located in the optimum band of R_m with a projection of 0.94. It is highlighted that the experimental results of SCM 17, which has the SCBA/s ratio of 25% and sp/c of 0.45, with G_m and R_m experimental values of 4.78 and 1, respectively, and although it has not been part of the analysis of the CCRD, present proximity to the prediction values that obtained the maximum performance. Above all, it is important to highlight that it was verified that the optimum values obtained in the G_m and R_m tests predict that it is possible, by the mathematical model, to reproduce the values satisfactorily, with a reliability higher than 99%.

4 Conclusions

In this paper, it was studied different compositions of SCM with the addition of SCBA by means of the relative flow area and relative flow time. The reference sample (SCM 0) had only sand as fine aggregate and required the same amount of superplasticizer as observed

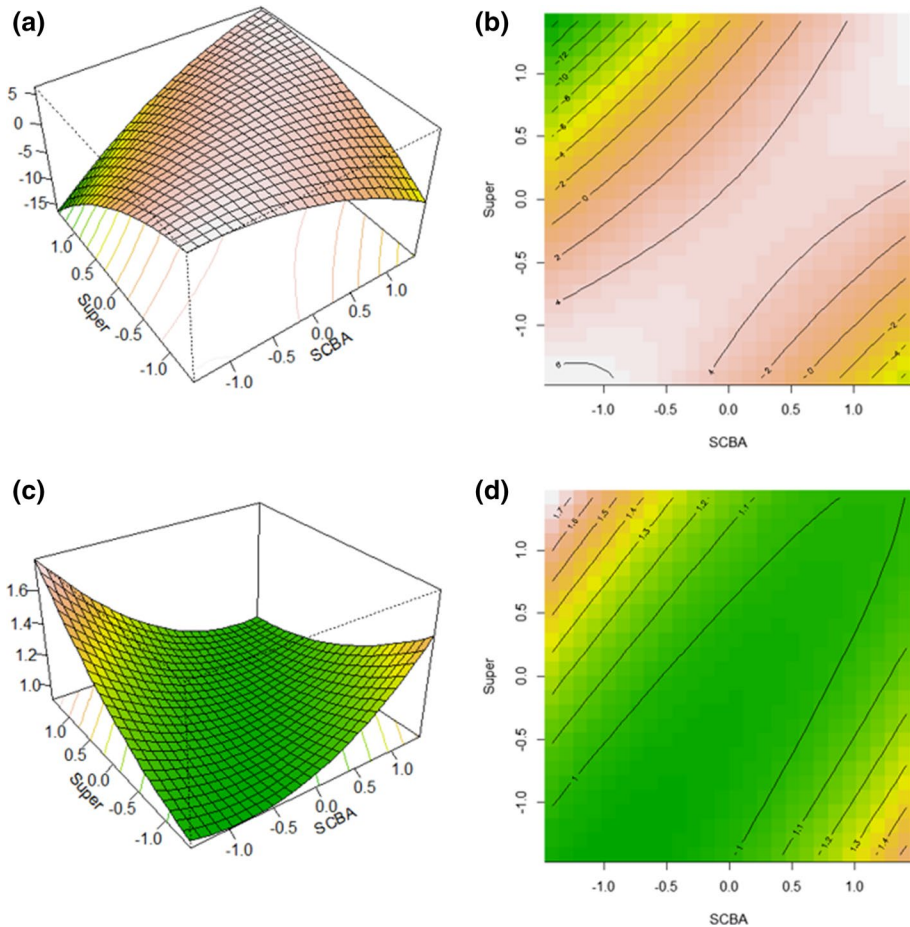


Fig. 8 Representation of the surface responses and respective contours of G_m and R_m . **a** Response surface graph for G_m ; **b** contour graph for G_m ; **c** response surface graph for R_m ; **d** contour graph for R_m

in the paste of Molin Filho et al. (2011) to maintain the expected rheological properties in the mortar. SCM 25, which represents the best composition for the replacement rate of 40% of sand by SCBA, although requested a $sp/c = 0.0058$ ratio, also presents interesting consumption values of materials for mortars with self-compacting properties.

It was demonstrated that it was possible to develop mortars with self-compacting properties with maximum sp/c consumption of 0.58% for the replacement rates between 5 and 40% of sand by SCBA. These percentages of replacement generated reduction in the cement and sand consumption per m^3 in the mortars in relation to the trace of mortar that did not contain SCBA. This reduction was of 11% (56 kg) for the cement and reached 46% (489 kg) in the case of the sand, for the replacement rate of 40%. SCM 25 composition would allow the production of more than 13 million m^3 of self-compacting concrete by using all the potential of SCBA generation of the annual harvest. This volume of SCC would be sufficient for the construction of more than 160 stadiums like the original project of Maracanã Olympic stadium. These values show an excellent alternative to the reuse of

the residues from the burning of the sugarcane bagasse, as much for the recycling as for the possible development of a product with quality comparable to those of current use. In addition, this application enables a suitable technical destination for SCBCA and has the potential to promote the deceleration of the use of natural resources, such as cement, sand and water in self-supporting mortars. In this way, advances are presented in the development of new products that directly contribute to the advancement of socio-environmental values, both for generating plants and for use in civil construction in favor of the advance of buildings. Although the statistical and rheological studies in the fresh state have shown substitution possibilities expressively greater than the usual compositions, new studies in the hardened state are recommended for the final proof of the proposed ideas.

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